

“Self Healing” Building

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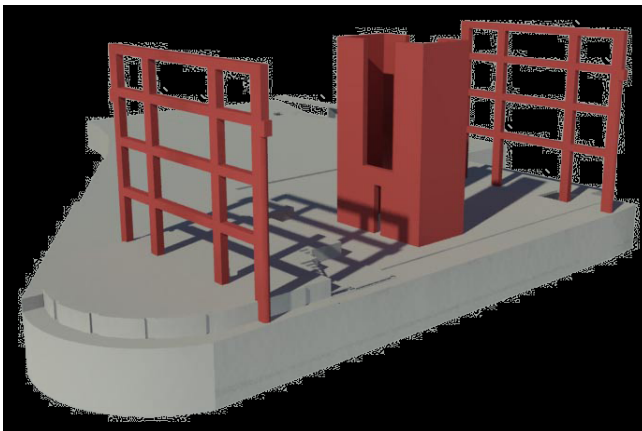
Concept: Elastic loading follows the same path on unloading, resulting in zero net strain. Inelastic loading results in a net strain and dissipated energy.

Structure: David Brower Center, Berkeley California

The David Brower Center is designed to withstand a 7.0+ magnitude earthquake which is predicted to occur within the next 30 years. “Continued functionality of the structure after a major earthquake... is a key “green” construction goal (Stevenson, Panian, Korolyk, & Mar, 2008). Buildings are designed for ULS during seismic events, but are rendered unserviceable by deflections. This building is designed to self-centre. Ground blast furnace slag was also used as a cementitious material, saving an estimated 5000 tons of CO₂ over the project. (Stevenson & Panian, Sustainability through strength, 2009)

Elastic and Inelastic response

The structure uses unbonded pre-stressed moment frames and core walls. During an earthquake, the pre-stressing cable remains elastic and acts to re-centre these members. Conventional steel reinforcement absorbs energy as it yields.



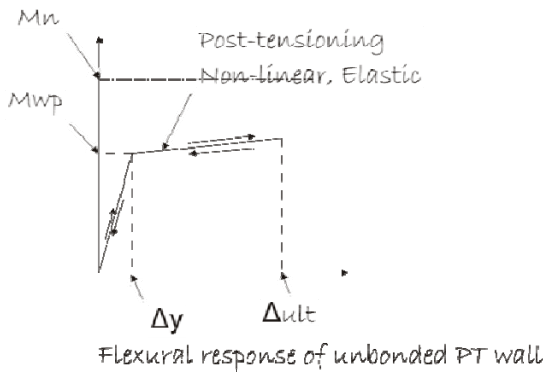
David Brower Center Moment frames and core walls.

Taken from (Stevenson, Panian, Korolyk, & Mar, 2008)

Hysteresis response of combined PT and CIP frame

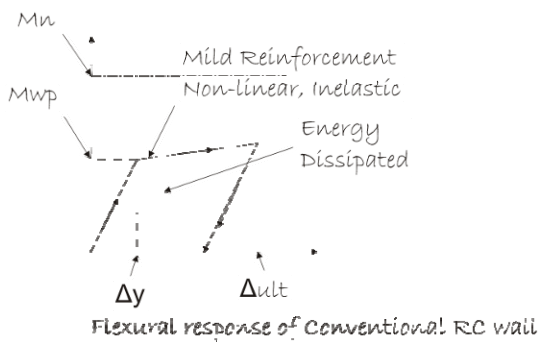
Figures adapted from (Stevenson, Panian, Korolyk, & Mar, 2008)

The energy dissipated is the area under the hysteresis curves. An elastic response means the member will follow the same load path during loading and unloading, resulting in no net strain. The inelastic response results in a large net displacement and a large dissipation of energy. The superimposed curve formed by the PT-RC wall encloses a large area with a small residual displacement. The idealised curves are shown below:



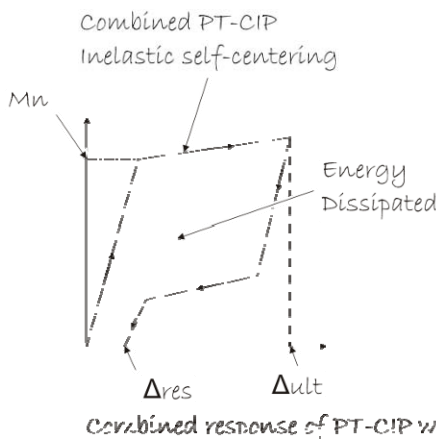
Unbonded PT wall:

- Elastic response
- No $\Delta_{residual}$
- No energy dissipated



Conventional RC wall:

- Inelastic response
- large $\Delta_{residual}$
- large energy dissipated

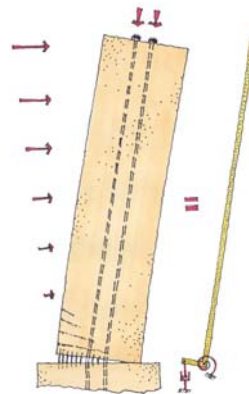


Combined PT-RC wall:

- Inelastic response
- small $\Delta_{residual}$
- large energy dissipated

Plastic hinge analysis

The reinforcing steel is concentrated in the outer part of the web, maximizing its strain. Strain is distributed over the cable length keeping the strand stress within the elastic region. If the moment due to the pre-stressed cable is greater than the plastic moment of hinge, then the joint will close after the earthquake. The concrete at the toe of the hinge must be strong enough to resist crushing.



PT-RC combined mechanism. taken from (Stevenson, Panian, Korolyk, & Mar, 2008)

Part B: Model

To demonstrate this concept, a model was constructed. Elastics were used to model the pre-stressing strand and chewing gum for the steel reinforcement.

A. Elastic only



Large displacement
Returned upright

B. Gum only



Stiffer structure

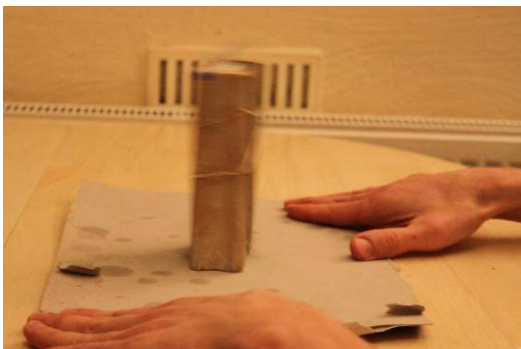


Did not return upright

C. Elastic and Gum



Stiffest structure



Returned upright

References

- Ji, T. (2009). Structural Concepts and finite element analysis of plane problems. Lecture Notes .
 Stevenson, M., & Panian, L. (2009). Sustainability through strength. Concrete international , 3-7.
 Stevenson, M., Panian, L., Korolyk, M., & Mar, D. (2008). Post-tensioned concrete walls and frames for seismic resistance. SEAOC 2008 conference proceedings (p. 8). Seismic engineering association California